

ANYTOWN FIELD HOUSE

ANYTOWN PUBLIC SCHOOLS - 2014 BOND 606 E. THIRD STREET, ANYTOWN, OK. 00000

ANYTOWN PUBLIC SCHOOL 2014 BOND - FIELD HOUSE

REVISIONS

REV DATE DESCRIPTION

1 06/03/2015 FIRE MARSHALL REVISIONS

PROJ. MANAGER:

DRAWN BY:

CHECKED BY:

MAY 11, 2015
PROJECT NO.:

1416

SHEET TITLE:

COVER SHEET

SHEET NO.:

TF-000

BUILDING CODE INFORMATION

TYPES OF CONSTRUCTION (IBC CHAPTER 6)

EGRESS WIDTH (IBC, SECTION	ON 1005)			
MINIMUM REQUIRED EGRES OF EGRESS MULTIPLIED BY		WIDTH OF MEANS O	F EGRESS IN INCHES NOT LESS THAN THE	TOTAL OCCUPANCT LOAD SERVED BY THE MEANS
OCCUPANCY OF 160 -> 160 *		VIDTH		
MEANS OF EGRESS ILLUMIN	IATION (IBC, SECTION 1006)			
EMERGENCY LIGHTING	AUTOMA	ΓIC	REQUIRED	
THE MEANS OF EGRESS, INC	CLUDING THE EXIT DISCHARC	SE, SHALL BE ILLUMI	NATED AT ALL TIMES THE BUILDING SPAC	E SERVED BY THE MEANS OF EGRESS IS OCCUPIED.
EXIT SIGNS (IBC, SECTION 1	•			
EXIT SIGNS	AUTOMA		REQUIRED	
EXITS AND EXIT ACCESS DO	ORS SHALL BE MARKED BY A	N APPROVED EXIT	SIGN READILY VISIBLE FROM ANY DIRECT	ON OF EGRESS TRAVEL.
EXIT ACCESS TRAVEL DISTA	ANCE (IBC, SECTION 1016, TA	BLE 1016.1)		
OCCUPANCY	COMPONENT	REQUIRED (F	SOURCE	PROVIDED
\wedge	COMMON PATH OF TRAV	EL 75	NFPA 101 14.2.5.3.2	67 FEET (WEIGHT ROOM NO. 111)
EDUCATIONAL E /1	DEAD END CORRIDORS	20	IBC 1016.3	0 FEET (NO DEAD END CORRIDORS IN EDUCATIONAL USE
	TRAVEL TO EXIT	150	NFPA 101 14.2.6.2	67 FEET (WEIGHT ROOM NO. 111)
Y Y Y	Y Y	γ · γ ·	<u> </u>	Y Y Y Y Y Y Y Y
	BC, SECTION 1024; ICC-300)			
ASSEMBLY - BLEACHERS (II	T IN THE BOOK INC.	L BE BY OWNER. BL	EACHER DESIGN AND PERMIT TO BE SUBI	IITTED AT A LATER DATE.
,	I IN THIS PROJECT AND WIL		ENOMER DEGICITATION ENTINE TO BE COB	
•	T IN THIS PROJECT AND WIL		ENONER BEGGN AND LEAVING TO BE GGB.	
•	OT IN THIS PROJECT AND WIL			
,	OT IN THIS PROJECT AND WIL	ACCE	ESSIBILITY FEATURE	<u> </u>
•	OT IN THIS PROJECT AND WIL	ACCE		<u> </u>
,	Y (IBC CHAPTI			<u>S</u>

	<u>A</u>	CCESSIBILITY FEATU	<u>RES</u>	
ACCESSIBILITY	(IBC CHAPTER 11)			
ACCESSIBLE PARKING SPACE	ES (IBC, TABLE 1106.1)			
PARKING REQUIREMENTS	REQ	UIRED	PROVI	DED
GROUP	BUILDING AREA	SPACES	BUILDING AREA	SPACES
EDUCATIONAL E	UP TO 12,000 SQ. FT. GLA	1 SPACE PER 200 SQ. FT. GLA	7,729 SQ. FT.	39 REQUIRED
	ACCESSIBIL	E REQUIRED	ACCESSIBILE	PROVIDED
	TOTAL SPACES	REQUIRED MINIMUM	TOTAL SPACES	SPACES
1	26 TO 50	2	39 TOTAL	2
	PLU	MBING SYSTEMS FEA	TURES	

PLUMBING SYSTEMS (IBC CHAPTER 29)

REQUIRED

		WATER	CLOSETS	1 41/4	TODICO	DATUTUDO	DDINIKINO	OTHER	1) -					
					TORIES	BATHTUBS OR SHOWER	DRINKING FOUNTAINS	OTHER		NTERIOR WALL AND CEILI	NG FINISH REQUIRE	EMENTS BY OCCU	JPANCY (IBC, TAB	LE 803.9)
CLASSIFICATION	OCCUPANCY	MALE	FEMALE	MALE	FEMALE							S	SPRINKLERED	
EDUCATIONAL E	155	1 PER 50 (3	REQUIRED)	1 PER 50 (3	REQUIRED)	-	1 PER 100	1 SERVICE SINK])	GROUP	EXITS		CORRIDORS	ROO
ASSEMBLY A-5 (FOR 2000 EXISTING AND NEW BLEACHERS)	2000	13 (1 PER 75 FOR THE FIRST 1,500 AND 1 PER 120 FOR THE REMAINDER EXCEEDING 1,500)	25 (1 PER 40 FOR THE FIRST 1,520 AND 1 PER 60 FOR THE REMAINDER EXCEEDING 1,500)	5 (1 PER 200)	7 (1 PER 150)	-	2 (1 PER 1000)	1 SERVICE SINK		EDUCATIONAL E				
REQ'D TOTALS	2155	46	25	0	40		2	2 SERVICE SINK		FIRE PROTEC	TION SYS	TEMS (IB	BC CHAPT	ER 9)
REQUIDIALS	2155	16	25	8	10	-	3	2 SERVICE SINK		AUTOMATIC SPRINKLER SY	YSTEMS (IBC, SECT	ION 903)		
EXISTING									+ $)$	STATUS	AUTOMATIC SF	PRINKLER SYSTEM	M NOT PROVIDED	
EXISTING		WATER	CLOSETS	1.43/4	TORIES	BATHTUBS	DRINKING	OTHER	$\mathbb{K} \square$	GROUP				
	COCUDANOV					OR SHOWER	FOUNTAINS	OTHER	\mathbb{K}	EDUCATIONAL E	PER SECTION 9	903.2.3, AN AUTON	MATIC SPRINKLER	SYSTEM IS NOT
CLASSIFICATION	OCCUPANCY	MALE	FEMALE	MALE	FEMALE				1)[·			
EDUCATIONAL E	4000	0 (4 - 0							P	PORTABLE FIRE EXTINGUIS	SHERS (IBC, SECTIO	ON 906)		
ASSEMBLY A-5	1000	6 (4 + 2 URINALS)	6	2	2	-	-	-	F	FIRE EXTINGUISHERS FOR	CLASS A FIRE HAZ	ARDS (IBC, TABL	E 906.3(1))	
(FOR 1000 EXISTING BLEACHERS)								-	\mathbb{Z}				ORDINARY (MODE	ERATE) HAZARC
,									\ \ \ \	MIN. RATED SINGLE EXTING	GUISHER			2-A
										MAXIMUM FLOOR AREA PER	R UNIT OF A			1,500 SQ. FT.
									1) [MAXIMUM FLOOR AREA FO	R EXTINGUISHER		1	11,250 SQ. FT.
PROVIDED									\\ \\ \\ \\	MAXIMUM TRAVEL DISTANC	CE TO EXTINGUISHE	ER .		75 FEET
-			CLOSETS		TORIES	BATHTUBS OR SHOWER	DRINKING FOUNTAINS	OTHER	\mathbb{K}			I		
CLASSIFICATION	OCCUPANCY	MALE	FEMALE	MALE	FEMALE					FIRE ALARM AND DETECTION	ON SYSTEMS (IBC, S	SECTION 907.2.3)		
EDUCATIONAL E	155	4 (1 + 2 URINAL)	3	4	3	-	2 (HI-LO)	1 SERVICE SINK		GROUP EDUCATIONAL E		·		
ASSEMBLY A-5	1000	8 (3 + 5 URINALS)	17	8	8	-	2 (HI-LO)	1 SERVICE SINK	K F	FIRE ALARM SYSTEM		MANUAL	RE	EQUIRED
(FOR 1000 EXISTING BLEACHERS)										SMOKE DETECTION SYSTEI	M	AUTOMATIC	RE	EQUIRED
belaciieks)									$K \vdash$					

L	
	PLUMBING SYSTEMS HAVE BEEN DESIGNED AND CALCULATED FOR THE FUTURE INSTALLATION OF ADDITIONAL BLEACHERS.

FIRE-RESISTANCE RATING REQUIREMENTS FOR BUILDING	ELEMENTS (IBC, TABLE 601)				
BUILDING ELEMENT	HOURLY RATING REQUIRED	HOURLY RATING PROVIDED	METHOD OF ACHIEVING RATII	NG	
PRIMARY STRUCTURAL FRAME (SEE IBC SECTION 202)	0	E 0	N/A		
EXTERIOR BEARING WALLS	0	4	8" CMU BLOCK -SOLID FILL CE	ILLS 1	
INTERIOR BEARING WALLS	0	4	8" CMU BLOCK - SOLID FILL CE	ELLS	
EXTERIOR NONBEARING WALLS	0	4	8" CMU BLOCK -SOLID FILL CE	ILLS	
INTERIOR NONBEARING WALLS	0	4	8" CMU BLOCK - SOLID FILL CE	ELLS	
FLOOR CONSTRUCTION AND SECONDARY MEMBERS	0	0	N/A		
ROOF CONSTRUCTION AND SECONDARY MEMBERS	0	1	BUILT-UP ROOF ON METAL DE	CK ON STEEL JOISTS	
				~ ~ ~ ~ ~	
FIRE-RESISTANCE RATING REQUIREMENTS FOR EXTERIO	R WALLS BASED ON FIRE SEPARA	TION DISTANCE (IBC, TABLE 602)		
FIRE SEPARATION DISTANCE = X (feet)	TYPE OF CONSTRUCTION	OCCUPANCY GROUP	FIRE RESISTANCE REQUIRED	FIRE RESISTANCE PROVIDE	
10 ≤ X ≤ 30	IIB	Е	0	4	
FIRE AND SMOKE PROTECTION	ON FEATURES (IB	C CHAPTER 7)			
FIDE AND SMOKE DROTECTION	NI FEATURES (IR	C CUARTER 7\			
MAXIUMUM AREA OF EXTERIOR WALL OPENINGS BASED	•		ECTION (IBC, TABLE 705.8)		
FIRE SEPARATION DISTANCE (feet)	DEGREE OF OPE	NING PROTECTION	ALLOWA	BLE AREA	
30 OR GREATER	UNPROTECTED, I	NONSPRINKLERED	NO LIMIT		
FIRE WALLS (IBC, SECTION 706)					
FIRE SEPARATION DISTANCE (feet)	DEGREE OF OPE	NING PROTECTION	ALLOWAI	BLE AREA	
30 OR GREATER	UNPROTECTED, I	NONSPRINKLERED	NO L	IMIT	
FIRE WALL FIRE-RESISTANCE RATINGS (IBC, TABLE 706.4)					
GROUP	FIRE RESISTANC	E RATING (hours)	CONTRUCT	TION NOTES	
E		2	NO FIRE WALL REQUIR	ED FOR THIS PROJECT	
FIRE-RESISTANCE RATING REQUIREMENTS FOR FIRE BAR	RIER ASSEMBLIES OR HORIZONTA	AL ASSEMBLIES BETWEEN FIRE	AREAS (IBC, TABLE 707.3.9)		
GROUP	FIRE RESISTANC	CE RATING (hours)	CONTRUCT	TION NOTES	
E		2	NO FIRE BARRIER REQU	IRED FOR THIS PROJECT	
SHAFT ENCLOSURES (IBC, SECTION 708; NFPA 101 8.6.5)	FIDE DEGICTANG	DE DATING (I	OONTOLIOT	TION NOTES	
GROUP		CE RATING (hours)		FION NOTES	
E		X	NO SHAFT ENGLOSU	RES IN THIS PROJECT	

OCCUPANO	Y LOAD, ME	ANS OF EGRE	SS AND PAS	SIVE FIRE PR	OTECTION FEA	ATURES
INTERIOR FINISHES (IBC CHAPTER 8)						
INTERIOR WALL AND CEILING	FINISH REQUIREMENTS I	BY OCCUPANCY (IBC, TABLE	803.9)			
		SPRINKLERED			NONSPRINKLERED	
GROUP	EXITS	CORRIDORS	ROOMS	EXITS	CORRIDORS	ROOMS
EDUCATIONAL E				CLASS A	CLASS B	CLASS C

\leq	AUTOMATIC SPRINKLER SYST	TEMS (IBC, SECTION 903)		
	STATUS	AUTOMATIC SPRINKLER SYS	TEM <u>NOT PROVIDED</u>	
\langle	GROUP			
<	EDUCATIONAL E	PER SECTION 903.2.3, AN AU	TOMATIC SPRINKLER SYSTEM IS NOT REQUIRED.	
)	PORTABLE FIRE EXTINGUISH	ERS (IBC, SECTION 906)		
	FIRE EXTINGUISHERS FOR CL	ASS A FIRE HAZARDS (IBC, TA	ABLE 906.3(1))	
$\langle \ \ $			ORDINARY (MODERATE) HAZARD OCCUPANCY	EXTINGUISHERS AS PROVIDED
<	MIN. RATED SINGLE EXTINGUI	ISHER	2-A	
	MAXIMUM FLOOR AREA PER U	JNIT OF A	1,500 SQ. FT.	8,090 SQ. FT. / 1,500 SQ. FT. = 6 EXTINGUISHERS MINIMUM
)	MAXIMUM FLOOR AREA FOR E	EXTINGUISHER	11,250 SQ. FT.	BUILDING AREA LESS THAN MAXIMUM FLOOR AREA

)			
<	FIRE ALARM AND DETECTION SYSTEMS (IBC,	SECTION 907.2.3)	
	GROUP EDUCATIONAL E		
1	FIRE ALARM SYSTEM	MANUAL	REQUIRED
	SMOKE DETECTION SYSTEM	AUTOMATIC	REQUIRED
Ž.	OCCUPANCY NOTIFICATION SYSTEM	AUTOMATIC	REQUIRED
$ \langle $	EXIT SIGNS	-	REQUIRED

	MEANS OF EGRESS	(IRC	CHAP	ΓFR 1	0
\mathcal{I}	MEANS OF EGRESS		CHAP		U

· ·
OCCUPANCT LOAD CALCULATIONS (IBC TABLE 1004.1.1

			OCCUPANCY AREAS	TABLE 1004.1.2 - MAXIMUM FLOOR ALLOWANCES		TABLE 1004.1.2 - MAXIMUM FLOOR ALLOWANCES
ROOM NO.	NAME	Occupancy	ROOM AREA	S.F. PER PERSON	Area Type	OCCUPANCY LOAD CALCULATED
000	TICKET BOOTH	E	49 SF	100	GROSS	1
100	CORRIDOR	E	231 SF	0	-	0
101	VARSITY LOCKER ROOM	E	706 SF	20	NET	35
102	TOILETS	E	263 SF	50	NET	5
103	ELEC	S-2	52 SF	0	-	0
104	JUNIOR VARSITY LOCKER ROOM	E	499 SF	20	NET	25
105	TOILETS	E	263 SF	50	NET	5
106	JANITOR	S-2	53 SF	0	-	0
107	EQUIPMENT STORAGE	S-2	245 SF	50	NET	5
108	COACHES OFFICE	E	744 SF	50	NET	15
109	COACHES DRESSING	E	119 SF	50	NET	2
110	TAPE ROOM	E	162 SF	50	NET	3
111	WEIGHT ROOM	E	1508 SF	50	NET	30
200	CONCESSION	E	365 SF	50	NET	7
201	CONCESSION STORAGE	S-2	146 SF	50	NET	2
202	ELEC	S-2	69 SF	0	-	0
204	MEN	E	372 SF	50	NET	7
205	WOMEN	E	578 SF	50	NET	12
206	FAMILY RESTROOM	E	73 SF	50	NET	1

ROJECT NAME ANYTO	DWN PUBLIC SCHOOLS - NEW FIELD HOUSE AND STEM CLASSROOM BUILDINGS
ROJECT LOCATION 606 E	. THIRD STREE, ANYTOWN, OK. 00000
ROJECT CONSTRUCTION PURPOSE NEW	CONSTRUCTION - FIELD HOUSE AND CLASSROOM BUILDING
JILDING OWNER	, SUPERINTENDENT, PUBLIC SCHOOLS, PH:
STIMATED PROJECT COST FIELD	HOUSE - \$1,816,791; STEM BUILDING - \$2,067,025; TOTAL - \$4,549,874
JTHORITY HAVING JURISDICTION OKLA	HOMA STATE FIRE MARSHAL'S OFFICE
ROJECT DESCRIPTION - SUMMARY	
ASSROOM BUILDING - NEW CLASSROOM BUILDING (9,389 SQ. FT.) CONSISTING OF FIVE CLASSROOM BUILDING (4,421 SQ. FT.) IS BEING CONSTRUCTED AS A SAFE ROOM WITH REINFORCED CMU WATERUCTURE AND METAL STUD FRAMING. EXTERIOR MATERIALS WILL CONSIST OF COMMON BRICK,	LLS AND CONCRETE LID. THE REMAINING PORTIONS OF THE BUILDING WILL BE STE
ELD HOUSE BUILDING - NEW FIELD HOUSE BUILDING (7,729 SQ. FT.) CONSISTING OF TWO LOCKERIESTROOMS FOR THE SPORTS ASSEMBLY BLEACHERS. THE FIELD HOUSE IS BEING CONSTRUCTED DISSIST OF SPLIT FACE CMU, COMMON BRICK AND EIFS. BUILDING IS NOT SPRINKLERED.	
TERNATES - ALL ALTERNATES HAVE BEEN ACCEPTED ALTERNATE #1 (FIELD HOUSE): ALTERNATE #1 - DEDUCT ALTERNATE CONSISTS OF ALL MATERIALS AND LABOR NECESSARY TO CONSTRUCT: 2) 8'x78' BREEZEWAY ON NORTH SIDE OF BUILDING 3) FIVE MASONRY COLUMNS 4) 8'x115' TPO ROOFING SYSTEM	
5) 8'x30' STANDING SEAM METAL ROOF TERNATE #2 (STEM BUILDING): ALTERNATE #2 - DEDUCT ALTERNATE	
CONSISTS OF ALL MATERIALS AND LABOR NECESSARY TO CONSTRUCT: 1) NORTH VESTIBULE	
TERNATE #3 (STEM BUILDING): ALTERNATE #3 - DEDUCT ALTERNATE CONSISTS OF ALL MATERIALS AND LABOR NECESSARY TO CONSTRUCT: 1) SOUTH VESTIBULE	
APPLICABLE DESIGN CRITERIA (APPLICABLE CO	•
NAME TATE CODE ADOPTION	ÉDITION
	2000
(INTERNATIONAL BUILDING CODE (IBC)	2009
(INTERNATIONAL EXISTING BUILDING CODE (IEBC)	2009
(INTERNATIONAL FIRE CODE (IFC)	2009
(INTERNATIONAL FUEL GAS CODE (IFGC)	2009
(INTERNATIONAL MECHANICAL CODE (IMC)	2009
(INTERNATIONAL PLUMBING CODE (IPC)	2009
NATIONAL ELECTRIC CODE (NEC)	2011
DITIONAL CODES	
FEMA 320: TAKING SHELTER FROM THE STORM, BUILDING A SAFE ROOM FOR YOUR HOME C	
FEMA 361: DESIGN AND CONSTRUCTION GUIDANCE FOR COMMUNITY SAFEROOMS	2008
ICC 500: STANDARD FOR THE DESIGN AND CONSTRUCTION OF STORM SHELTERS	2008
NFPA 1: FIRE CODE	2012
NFPA 10: STANDARD FOR PORTABLE FIRE EXTINGUISHERS	2012
NFPA 13: STANDARD FOR THE INSTALLATION OF SPRINKLER SYSTEMS	2013
NFPA 24: STANDARD FOR THE INSTALLATION OF PRIVATE FIRE SERVICE MAINS AND THEIR A	
NFPA 30: FLAMMABLE AND COMBUSTIBLE LIQUIDS CODE	2012
NFPA 33: STANDARD FOR SPRAY APPLICATION USING FLAMMABLE OR COMBUSTIBLE MATER	
NFPA 70: NATIONAL ELECTRIC CODE	2011
NFPA 72: NATIONAL FIRE ALARM AND SIGNALING CODE	2013
NFPA 220: STANDARD ON TYPE OF BUILDING CONSTRUCTION	2012
	16 AUG 2010 WITH CHANGE 2, 28 NOV 2011
UNIFIED FACILITIES CRITERIA (UFC) 1-200-01: GENERAL BUILDING REQUIREMENTS	
UNIFIED FACILITIES CRITERIA (UFC) 1-200-01: GENERAL BUILDING REQUIREMENTS UNIFIED FACILITIES CRITERIA (UFC) 3-600-01: FIRE PROTECTION ENGINEERING FOR FACILITI	DR BUILDINGS 8 OCT 2003 INCLUDING CHANGE 1, 22 JAN 2007
UNIFIED FACILITIES CRITERIA (UFC) 1-200-01: GENERAL BUILDING REQUIREMENTS UNIFIED FACILITIES CRITERIA (UFC) 3-600-01: FIRE PROTECTION ENGINEERING FOR FACILITI UNIFIED FACILITIES CRITERIA (UFC) 4-010-01: DOD MINIMUM ANTITERRORISM STANDARDS FOR	
UNIFIED FACILITIES CRITERIA (UFC) 1-200-01: GENERAL BUILDING REQUIREMENTS UNIFIED FACILITIES CRITERIA (UFC) 3-600-01: FIRE PROTECTION ENGINEERING FOR FACILITI	MS 9 APR 2008 WITH CHANGE 1, JAN 2010
UNIFIED FACILITIES CRITERIA (UFC) 1-200-01: GENERAL BUILDING REQUIREMENTS UNIFIED FACILITIES CRITERIA (UFC) 3-600-01: FIRE PROTECTION ENGINEERING FOR FACILITI UNIFIED FACILITIES CRITERIA (UFC) 4-010-01: DOD MINIMUM ANTITERRORISM STANDARDS FOR UNIFIED FACILITIES CRITERIA (UFC) 4-021-01: DESIGN AND O&M: MASS NOTIFICATION SYSTEM	MS 9 APR 2008 WITH CHANGE 1, JAN 2010
UNIFIED FACILITIES CRITERIA (UFC) 1-200-01: GENERAL BUILDING REQUIREMENTS UNIFIED FACILITIES CRITERIA (UFC) 3-600-01: FIRE PROTECTION ENGINEERING FOR FACILITI UNIFIED FACILITIES CRITERIA (UFC) 4-010-01: DOD MINIMUM ANTITERRORISM STANDARDS FOR	MS 9 APR 2008 WITH CHANGE 1, JAN 2010
UNIFIED FACILITIES CRITERIA (UFC) 1-200-01: GENERAL BUILDING REQUIREMENTS UNIFIED FACILITIES CRITERIA (UFC) 3-600-01: FIRE PROTECTION ENGINEERING FOR FACILITI UNIFIED FACILITIES CRITERIA (UFC) 4-010-01: DOD MINIMUM ANTITERRORISM STANDARDS FOR UNIFIED FACILITIES CRITERIA (UFC) 4-021-01: DESIGN AND O&M: MASS NOTIFICATION SYSTEM	9 APR 2008 WITH CHANGE 1, JAN 2010

LIFE SAFETY AND FIRE PROTECTION CODE COMPLIANCE APPROACH NARATIVE

SE AND OCCUPANCY	CLASSIFICATION (IBC CHAPTER 3)	

	BUILDING NAME	IBC CLASSIFICATION	NFPA 101 CLASSIFICATION	AREA	OCCUPANCY LOAD
	NEW FIELD HOUSE	EDUCATIONAL E (SECTION 305)	NEW EDUCATIONAL OCCUPANCIES (CHAPTER 14)	7,729 SQ. FT. GROSS	
-				6,498 SQ. FT. NET	155

SPECIAL REQUIREMENTS BASED ON USE AND OCCUPANCY (IBC CHAPTER 4)

SENERAL BUILDING HEIGHTS AND AREA (IBC	CHAPTER 5)
LOWARD FRUIT DING UFFICITO AND AREAS (IDO TARI F 500)	AO DEGIGNED DUM DING HEIGH

	ALLOWABLE	BUILDING HEIGH	ITS AND AREAS (IBC, TABLE 503)	AS-DESIGNE	D BUILDING HEI	GHTS AND AREAS
			TYPE II B			TYPE II B
_		HEIGHT (ft)	55		HEIGHT (ft)	25
1	GROUP			GROUP		
	E	STORIES	2	E	STORIES	1
+		AREA	14,500 SF		AREA	7,729 SF
4						

BUILDING AREA MODIFICATIONS (IBC, SECTION 506)

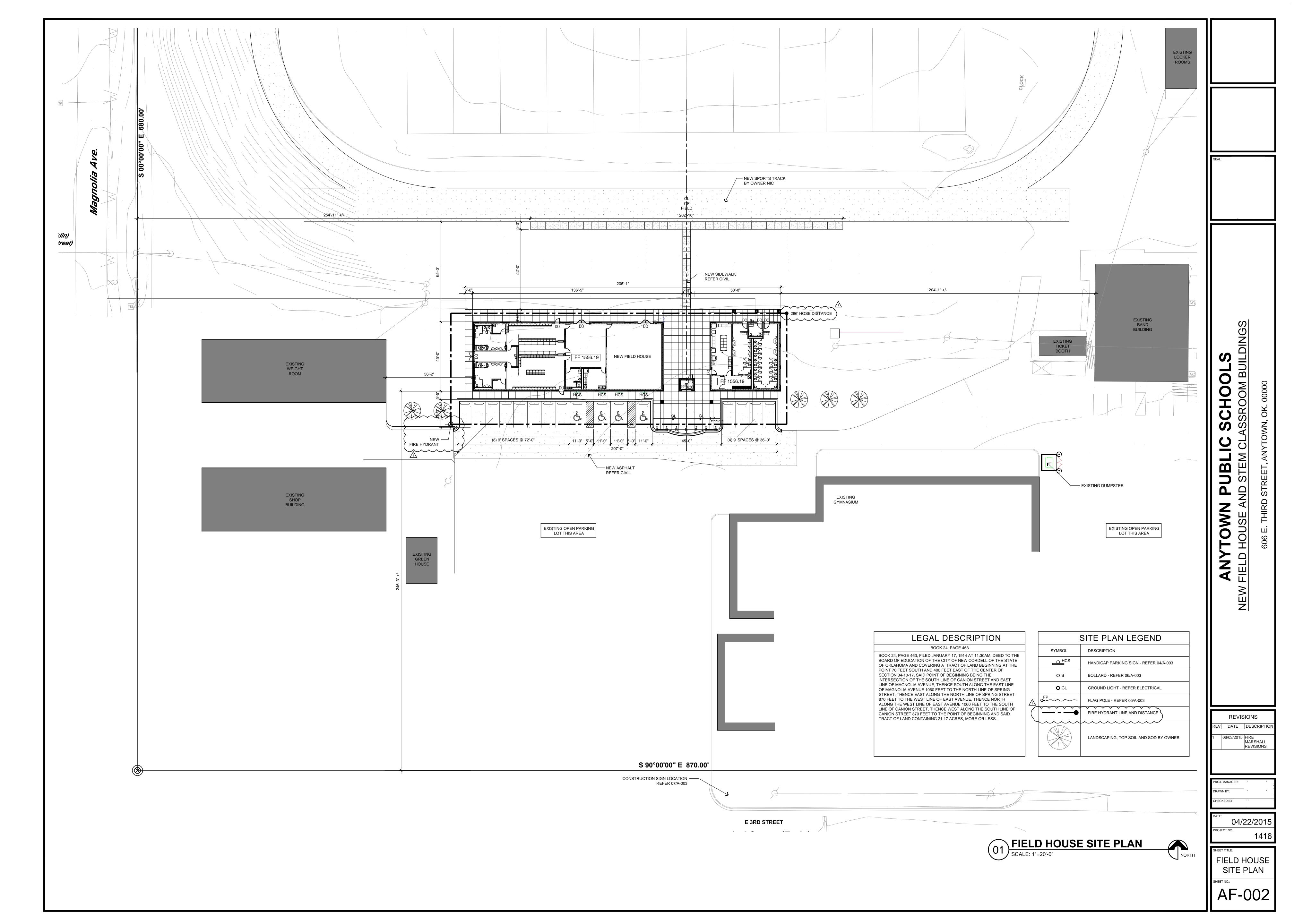
THE BUILDING AREAS LIMITED BY TABLE 503 SHALL BE PERMITTED TO BE INCREASED DUE TO FRONTAGE (If) AND AUTOMATIC SPRINKLER SYSTEM PROTECTION (I) IN ACCORDANCE WITH THE FOLLOWING: Aa = At + [At x If / 100] + [At x Is / 100]

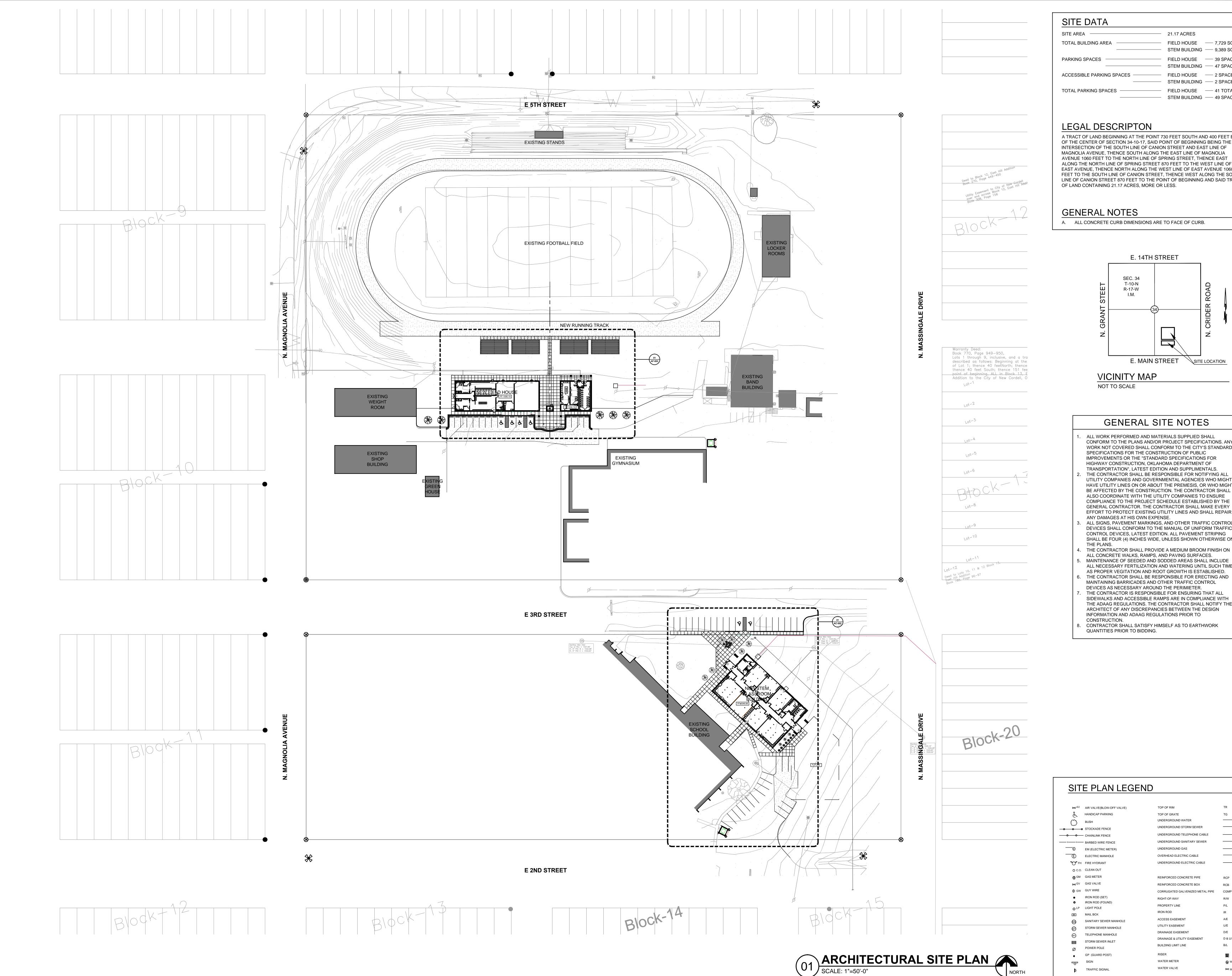
BUILDING AREA MODIFICATIONS ARE NOT REQUIRED OR APPLICABLE SINCE THE PROJECT SCOPE OF WORK COMPLIES WITH THE ALLOWABLE BUILDING HEIGHT AND AREA FROM TABLE 503.

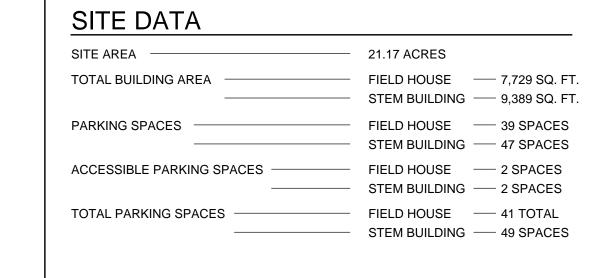
REQUIRED SEPARATION OF OCCUPANCIES (IBC, SECTION 508)

SEPARATION OF OCCUPANCIES ARE NOT REQUIRED OR APPLICABLE SINCE THE PROJECT SCOPE OF WORK CONSISTS OF ONE OCCUPANCY TYPE.

LIFE SAFETY AND INFORMATION





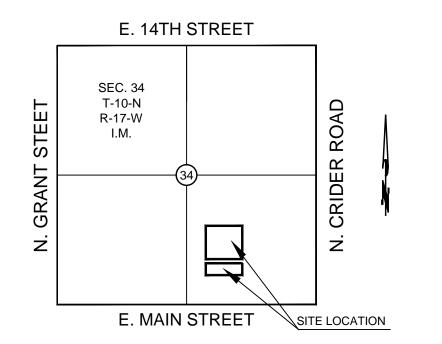


LEGAL DESCRIPTON

A TRACT OF LAND BEGINNING AT THE POINT 730 FEET SOUTH AND 400 FEET EAST OF THE CENTER OF SECTION 34-10-17, SAID POINT OF BEGINNING BEING THE INTERSECTION OF THE SOUTH LINE OF CANION STREET AND EAST LINE OF MAGNOLIA AVENUE, THENCE SOUTH ALONG THE EAST LINE OF MAGNOLIA AVENUE 1060 FEET TO THE NORTH LINE OF SPRING STREET, THENCE EAST ALONG THE NORTH LINE OF SPRING STREET 870 FEET TO THE WEST LINE OF EAST AVENUE, THENCE NORTH ALONG THE WEST LINE OF EAST AVENUE 1060 FEET TO THE SOUTH LINE OF CANION STREET, THENCE WEST ALONG THE SOUTH LINE OF CANION STREET 870 FEET TO THE POINT OF BEGINNING AND SAID TRACT OF LAND CONTAINING 21.17 ACRES, MORE OR LESS.

GENERAL NOTES

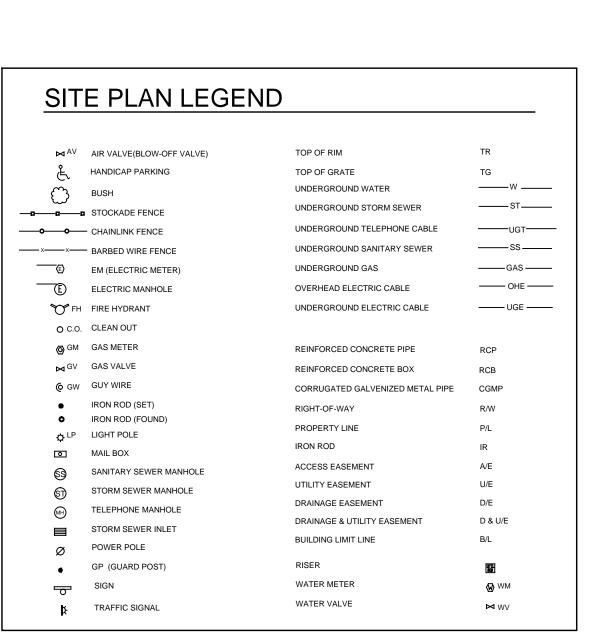
A. ALL CONCRETE CURB DIMENSIONS ARE TO FACE OF CURB.



VICINITY MAP NOT TO SCALE

GENERAL SITE NOTES

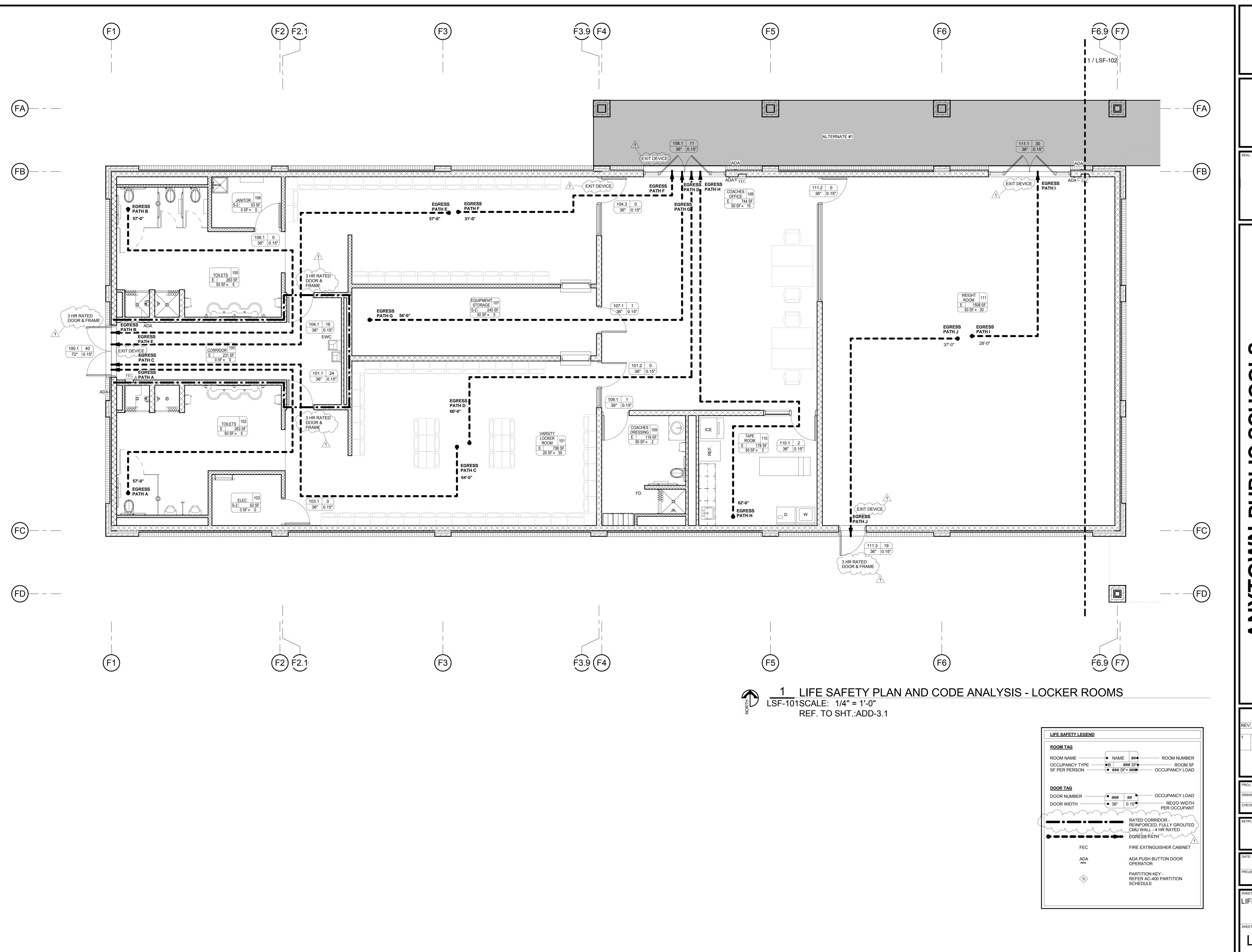
- ALL WORK PERFORMED AND MATERIALS SUPPLIED SHALL CONFORM TO THE PLANS AND/OR PROJECT SPECIFICATIONS. ANY WORK NOT COVERED SHALL CONFORM TO THE CITY'S STANDARD SPECIFICATIONS FOR THE CONSTRUCTION OF PUBLIC IMPROVEMENTS OR THE "STANDARD SPECIFICATIONS FOR
- TRANSPORTATION", LATEST EDITION AND SUPPLIMENTALS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING ALL UTILITY COMPANIES AND GOVERNMENTAL AGENCIES WHO MIGHT HAVE UTILITY LINES ON OR ABOUT THE PREMESIS, OR WHO MIGHT BE AFFECTED BY THE CONSTRUCTION. THE CONTRACTOR SHALL ALSO COORDINATE WITH THE UTILITY COMPANIES TO ENSURE COMPLIANCE TO THE PROJECT SCHEDULE ESTABLISHED BY THE GENERAL CONTRACTOR. THE CONTRACTOR SHALL MAKE EVERY
- EFFORT TO PROTECT EXISTING UTILITY LINES AND SHALL REPAIR ANY DAMAGES AT HIS OWN EXPENSE. ALL SIGNS, PAVEMENT MARKINGS, AND OTHER TRAFFIC CONTROL DEVICES SHALL CONFORM TO THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES, LATEST EDITION. ALL PAVEMENT STRIPING SHALL BE FOUR (4) INCHES WIDE, UNLESS SHOWN OTHERWISE ON THE PLANS.
- THE CONTRACTOR SHALL PROVIDE A MEDIUM BROOM FINISH ON ALL CONCRETE WALKS, RAMPS, AND PAVING SURFACES. MAINTENANCE OF SEEDED AND SODDED AREAS SHALL INCLUDE ALL NECESSARY FERTILIZATION AND WATERING UNTIL SUCH TIME AS PROPER VEGITATION AND ROOT GROWTH IS ESTABLISHED.
- MAINTAINING BARRICADES AND OTHER TRAFFIC CONTROL DEVICES AS NECESSARY AROUND THE PERIMIETER. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT ALL SIDEWALKS AND ACCESSIBLE RAMPS ARE IN COMPLIANCE WITH THE ADAAG REGULATIONS. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BETWEEN THE DESIGN
- CONSTRUCTION. CONTRACTOR SHALL SATISFY HIMSELF AS TO EARTHWORK QUANTITIES PRIOR TO BIDDING.



REVISIONS REV. DATE DESCRIPTION

04/22/2015

OVERALL SITE PLAN



ANYTOWN PUBLIC SCHOOLS
2014 BOND - FIELD HOUSE

REVISIONS

V DATE DESCRIPTION

06/03/2015 FIRE MARSHALL REVISIONS

J. MANAGER:

WN BY:

CKED BY:

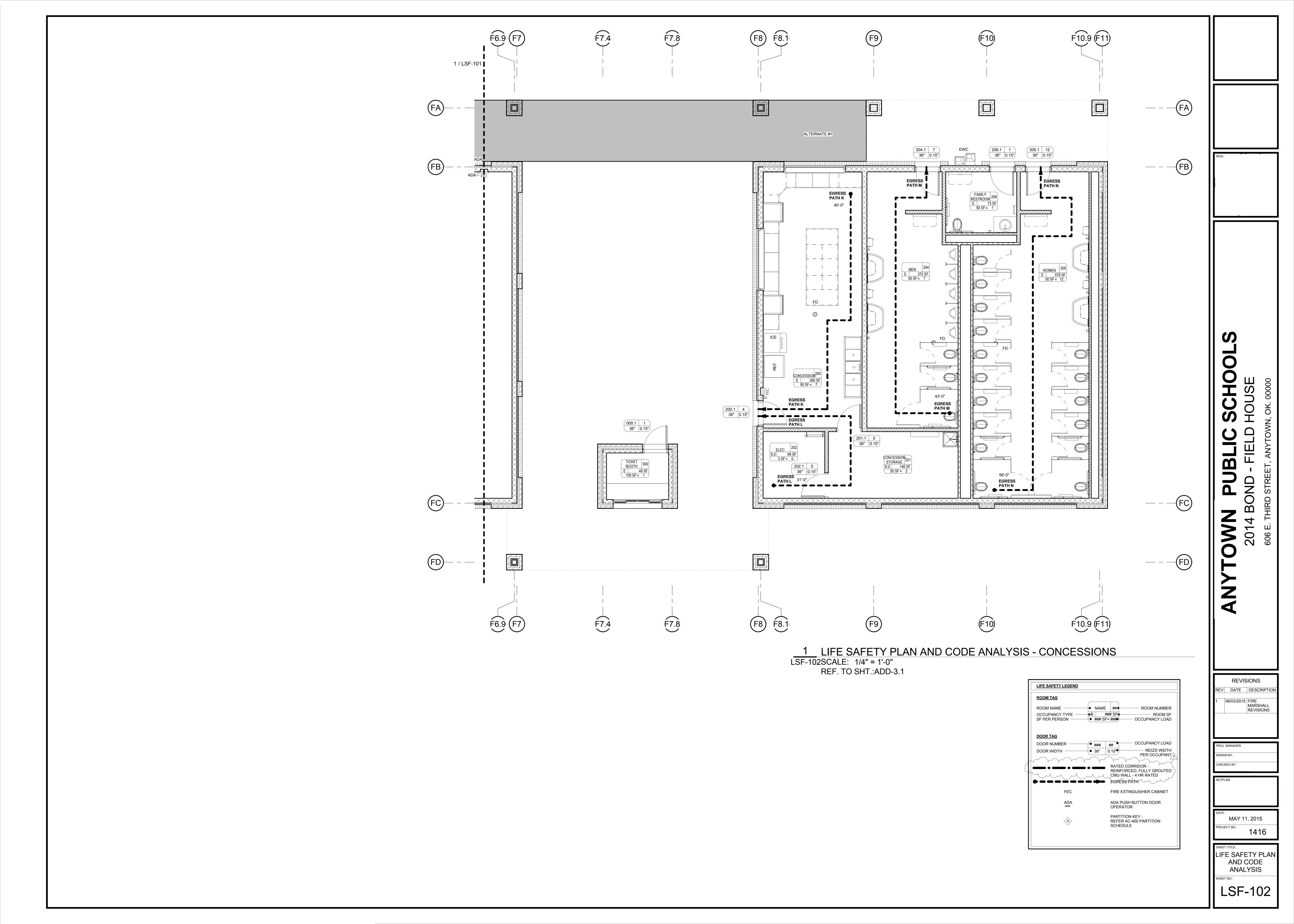
MAY 11, 2015

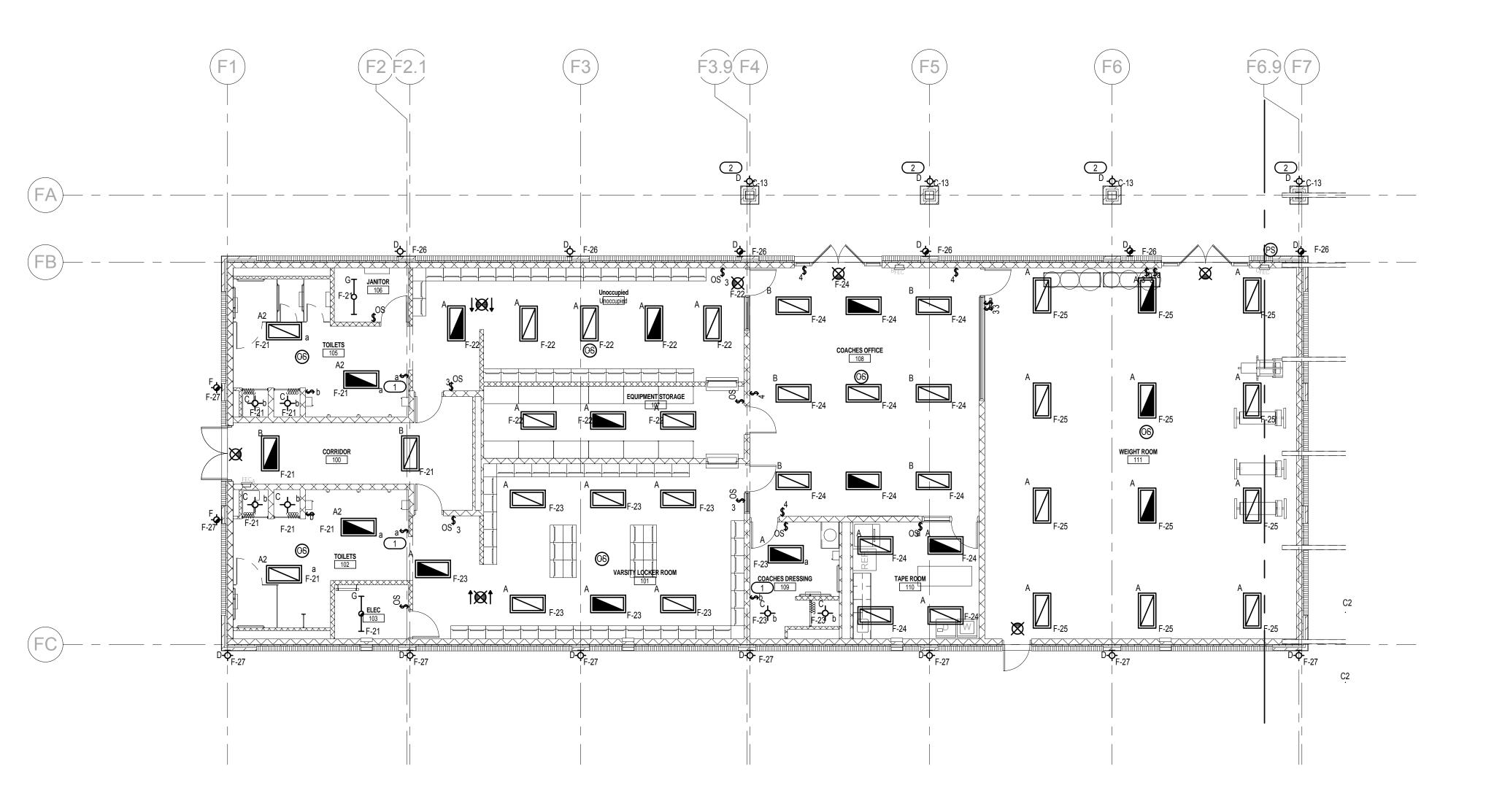
DJECT NO.:

1416

LIFE SAFETY PLAN
AND CODE
ANALYSIS

LSF-101





1 LIGHTING PLAN - LOCKER ROOMS

KEYED NOTES

1 EXHAUST FAN SHALL BE CONTROLLED FROM SWITCH CONTROLLING LIGHTS ON CIRCUIT

2 FIXTURES SHALL BE ON CIRCUIT FROM CONCESSION BUILDING. PILLARS AND FIXTURES ARE PART OF DEDUCT ALTERNATE.

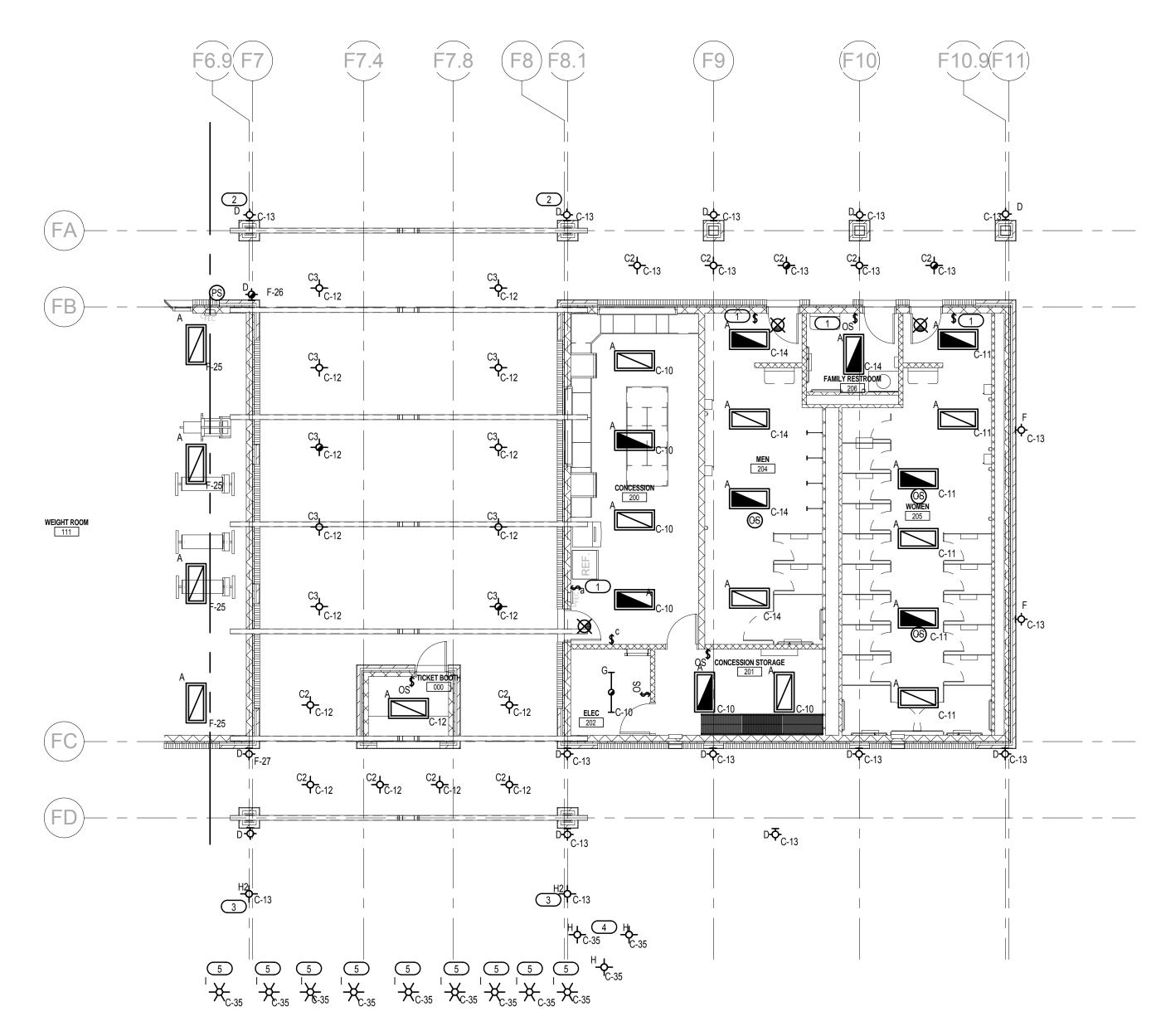
3 UPLIGHT FOR SIGNAGE. VERIFY WITH ARCHITECT FOR EXACT LOCATION.

4 UPLIGHT FOR FLAG. VERIFY WITH ARCHITECT FOR EXACT LOCATION. FIXTURES. SHALL BE PLACED AT 1/3 OF HEIGHT OF POLE AWAY FROM BASE OF POLE. FIXTURES SPACED EVENLY AROUND POLE. POINT FIXTURE DIRECTLY AT FLAG.

5 BOLLARD FIXTURES. VERIFY WITH ARCHITECT FOR EXACT LOCATION.

GENERAL NOTES

1. ALL OUTDOOR LIGHTING SHALL BE RAN FROM PHOTOCELL/TIMECLOCK. SEE DETAIL.



2 LIGHTING PLAN - CONCESSIONS

1/8" = 1'-0"

ANTIOWN POBLIC SCHOOLS

NEW FIELD HOUSE AND STEM CLASSROOM BUILDING

REVISIONS
REV. DATE DESCRIPTION

PROJ. MANAGER:
DRAWN BY:
CHECKED BY:

EYPLAN:

APR. 22, 2015
PROJECT NO.:

1416

ELECTRICAL LIGHTING PLAN

EF-101

	DESIGN PARAMETERS (EXCLUDING	SAFEROOM)
1.	BUILDING CODE LOCAL AMENDMENTS OCCUPANCY CATEGORY	IBC 2009 N/A STRUCTURAL III
2.	LIVE LOADS (UNIFORM/CONCENTRATED) A. ROOF	20 PSF / 300 LB
3.	ROOF SNOW LOAD A. GROUND SNOW LOAD, Pg B. FLAT ROOF SNOW LOAD, Pf C. SNOW EXPOSURE FACTOR, Ce D. SNOW LOAD IMPORTANCE FACTOR, I E. THERMAL FACTOR, Ct	10 PSF 7 PSF 1.0 1.0 1.0
4.	WIND DESIGN DATA A. BASIC WIND SPEED (3 SECOND GUST), V B. WIND IMPORTANCE FACTOR, I C. WIND EXPOSURE CATEGORY D. INTERNAL PRESSURE COEFFICIENT, GCpi E. DESIGN WIND PRESSURE ON COMPONENTS AND CLADDING 1.) WALLS (60 SQUARE FEET EFFECTIVE WIND AREA) END ZONES INTERIOR ZONES	90 MPH 1.15 C +/- 0.18 20.5 PSF 18 PSF
	2.) PARAPETS (10 SQUARE FEET EFFECTIVE WIND AREA)END ZONESINTERIOR ZONES3.) ROOF NET UPLIFT (340 SQUARE FEET EFFECTIVE WIND AREA)	65 PSF 48 PSF
	CORNER ZONES END ZONES INTERIOR ZONES F. WIDTH OF END ZONE	16 PSF 16 PSF 12 PSF 5.4 FT
5.	EARTHQUAKE DESIGN DATA A. SEISMIC IMPORTANCE FACTOR, I B. MAPPED SPECTRAL RESPONSE ACCELERATION, Ss C. MAPPED SPECTRAL RESPONSE ACCELERATION, S1 D. SITE CLASS E. SPECTRAL RESPONSE COEFFICIENT, Sds F. SPECTRAL RESPONSE COEFFICIENT, Sd1 G. SEISMIC DESIGN CATEGORY H. STRUCTURAL SYSTEM 1.) BASIC SEISMIC FORCE-RESISTING SYSTEM TYPE	1.25 20.8% 7.3% C .166 .083 B
	2.) VERTICAL ELEMENT TYPE 3.) SEISMIC RESPONSE COEFFICIENT, Cs 4.) RESPONSE MODIFICATION FACTOR, R J. ANALYSIS PROCEDURE	INTERMEDIATE SHEAR WALLS .104 2 EQUIVALENT LATERAL FORCE
	GENERAL NOTES	
	GENERAL	
1.	STRUCTURAL ELEMENTS ARE NON-SELF SUPPORTING AND REQUIRE IN ELEMENTS FOR STABILITY AND RESISTANCE TO LATERAL FORCES. FRA TEMPORARILY BRACED BY THE CONTRACTOR UNTIL PERMANENT BRACE AND WALLS HAVE BEEN INSTALLED AND CONNECTIONS BETWEEN THES	MING AND WALLS SHALL BE ING, FLOOR AND ROOF DECKS,
2.	THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE A METHOD OF CONSTRUCTION, UNLESS NOTED OTHERWISE. THE CONTR FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATIC SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.	ACTOR IS SOLELY RESPONSIBLE
3.	THE SIZE AND LOCATION OF EQUIPMENT PADS AND PENETRATIONS THE MECHANICAL, ELECTRICAL, AND PLUMBING WORK SHALL BE VERIFIED E PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE ENGINEER-O MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR OPENING LOSTRUCTURAL DRAWINGS.	BY THE CONTRACTOR. F-RECORD. REFER TO
4. 5. 6.	USE ONLY DIMENSIONS INDICATED IN THE CONTRACT DOCUMENTS. DO DOCUMENTS OR USE ANY DIMENSIONS TAKEN FROM ELECTRONIC DRAY ASSUME EQUAL SPACING IF NOT INDICATED IN CONTRACT DOCUMENTS THE SPECIFICATIONS ARE AN INTEGRAL PART OF THE CONTRACT DOCUCONJUCTION WITH THE CONTRACT DRAWINGS. WHERE REQUIREMENTS DRAWINGS DIFFER FROM THE SPECIFICATIONS, NOTIFY THE ARCHITECT ENGINEER-OF-RECORD.	WING FILES. S. JMENTS AND SHALL BE USED IN S INDICATED ON THE CONTRACT
7. 8. 9.	ARCHITECTURAL, MECHANICAL AND ELECTRICAL COMPONENTS AND SY CONSTRUCTED TO RESIST SEISMIC FORCES AS DETERMINED IN CHAPT THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ADEQUATE TEMPO OF EXISTING STRUCTURE DURING ALL PHASES OF CONSTRUCTION. DURING WELDING OR ANY OTHER CONSTRUCTION ACTIVITY THAT GENE HEAT, THE CONTRACTOR SHALL PROVIDE ADEQUATE FIRE PROTECTION AND CONTENTS.	ER 13 OF ASCE 7. PRARY SUPPORT AND STABILITY ERATES SPARKS OR INTENSE
	FOUNDATIONS	
1.	FOUNDATION DESIGNS, SUBGRADE PREPARATION NOTES, AND STRUCT	URAL EARTH MOVING

1.	FOUNDATION DESIGNS, SUBGRADE PREPARATION NOTES, AND STRUCTURAL EARTH MOVING
	SPECIFICATION ARE BASED ON THE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT
	NUMBER 731-14148A AND 731-14148B, BY: BURGESS ENGINEERING AND TESTING, INC. DATED: OCTOBER
	29. 2014.

FOOTING DESIGNS ARE BASED ON A MAXIMUM ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF FOR COLUMN FOOTINGS AND 1,400 PSF FOR WALL FOOTINGS.

CONTRACTOR AND TESTING LABORATORY REPRESENTATIVE SHALL READ THE GEOTECHNICAL REPORT AND BECOME THOROUGHLY FAMILIAR WITH SITE AND SUBGRADE INFORMATION GIVEN THEREIN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING EXACT QUANTITIES OF CUT AND FILL FOR ESTIMATING AND CONSTRUCTION. SUBGRADE SHALL BE PREPARED AS NOTED IN THE STRUCTURAL EARTH MOVING SPECIFICATION.

4. A QUALIFIED AND REGISTERED GEOTECHNICAL ENGINEER, LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED AND WORKING FOR THE TESTING LABORATORY, SHALL DETERMINE CONFORMANCE OF THE FOUNDATION BEARING STRATA WITH THE FOUNDATION DESIGN CRITERIA ABOVE, AND ALL OTHER CONTRACT DOCUMENTS. TESTING LABORATORY SHALL NOTIFY CONTRACTOR, ARCHITECT AND ENGINEER-OF-RECORD OF ANY CONDITIONS NOT IN ACCORDANCE WITH FOUNDATION DESIGN CRITERIA OR CONTRACT DOCUMENTS.

USE ONLY STRUCTURAL FILL MATERIAL AS NOTED IN THE STRUCTURAL EARTH MOVING SPECIFICATION FOR FILL BELOW BUILDING AND FIVE FEET BEYOND THE EDGES OF THE BUILDING.

EXTERIOR FOOTINGS SHALL BEAR AT OR BELOW MINIMUM BEARING DEPTH. MINIMUM BEARING DEPTH IS 24 INCHES BELOW ADJACENT FINISHED GRADE. THICKENED SLAB EDGE FOR STOOPS, CANOPIES, ETC. SHALL EXTEND 24 INCHES BELOW GRADE UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS.

FOUNDATION WALLS SHALL HAVE ADEQUATE TEMPORARY BRACING INSTALLED BY THE CONTRACTOR BEFORE BACKFILL IS PLACED AGAINST THEM. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL

WALL IS PERMANENTLY BRACED.

AVOID DAMAGE TO UNDERGROUND UTILITIES INCLUDING, BUT NOT LIMITED TO, WATER MAINS, SANITARY SEWERS AND BURIED CABLES WHICH MIGHT EXTEND ACROSS OR ADJOIN SITE.

I. MINIMUM COMPRESSIVE STRENGTH (f'c) AT THE END OF 28 DAYS SHALL BE AS FOLLOWS: A. FOOTINGS 3000 PSI B. FOUNDATION WALLS AND PEDESTALS 4000 PSI

REFER TO SPECIFICATIONS FOR MAXIMUM WATER/CEMENT RATIOS, MINIMUM CEMENT CONTENTS AND OTHER MIX DESIGN REQUIREMENTS. CONCRETE SHALL BE NORMAL WEIGHT (145 PCF), UNLESS NOTED OTHERWISE.

2. EXTERIOR CONCRETE AND CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL BE AIR-ENTRAINED. REFER TO CAST-IN-PLACE CONCRETE SPECIFICATION FOR AIR CONTENT.

MATERIALS OR ADMIXTURES SHALL NOT CONTAIN ANY CALCIUM CHLORIDE.

REINFORCING STEEL SHALL MEET THE FOLLOWING:
A. DEFORMED BARS
ASTM A615, GRADE 60
B. WELDABLE DEFORMED BARS
ASTM A706, GRADE 60

C. SLABS-ON-GRADE

B. WELDABLE DEFORMED BARS ASTM A706, GRADE 60
C. WELDED WIRE FABRIC ASTM A185
D. STEEL FIBERS ASTM A820
WHERE DOWELS ARE INDICATED BUT NOT SIZED, PROVIDE DOWELS THAT MATCH SIZE AND LOCATION OF

MAIN REINFORCING STEEL AND LAP SPLICE WITH THE MAIN REINFORCING STEEL. REINFORCING BARS SHALL BE SPLICED AS NOTED IN THE REINFORCING LAP SCHEDULE.
 REFER TO ACI 318 LATEST EDITION FOR CONCRETE COVER, ACI 315 LATEST EDITION FOR DETAILING PRACTICE AND FARRICATION, AND ACI 301 LATEST EDITION FOR STANDARD PRACTICE FOR MIXING AND

PLACING CONCRETE.

7. "C.J." INDICATES SAW CUT CONTRACTION JOINT OR DOWELED CONSTRUCTION JOINT IN SLAB-ON-GRADE. REFERENCE CAST-IN-PLACE CONCRETE SPECIFICATION FOR ACCEPTED SAW CUT METHODS. SLAB POURS SHALL BE SEPARATED BY A DOWELED CONSTRUCTION JOINT. CONTRACTION/CONSTRUCTION JOINTS SHALL BE LOCATED AS SHOWN ON PLANS OR AS DIRECTED BY THE ENGINEER-OF-RECORD.

PROVIDE CORNER BARS THAT MATCH AND LAP CONTINUOUS REINFORCEMENT SIZE AND QUANTITY AT

INTERSECTIONS AND CORNERS OF WALLS AND FOUNDATIONS.

ANCHORS INSTALLED IN HARDENED CONCRETE SHALL ONLY BE USED WHERE SPECIFIED ON THE CONTRACT DRAWING. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REINFORCING. HOLES SHALL BE DRILLED, DRY AND CLEANED AND ANCHORS INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED WRITTEN INSTRUCTIONS AND APPLICABLE ESR REPORT. REFERENCE DETAILS FOR ANCHOR SIZE AND EMBEDMENT. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED ON THE CONTRACT DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY THE PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION AND LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

11. INCLUDE AN ALLOWANCE IN THE BID PRICE FOR 1000 POUNDS OF REINFORCING STEEL TO BE FABRICATED AND PLACED AS DIRECTED BY THE ARCHITECT OR ENGINEER. ALLOWANCE IS TO INCLUDE, BUT IS NOT LIMITED TO: MATERIAL, DETAILING, FABRICATION, SHIPPING, INSTALLATION, OVERHEAD AND PROFIT

GENERAL NOTES

CONCRETE MASONRY UNITS SHALL MEET ASTM SPECIFICATION C90, GRADE N TYPE I BLOCK WITH A MINIMUM UNIT COMPRESSIVE STRENGTH = 1900 PSI. THE SPECIFIED DESIGN COMPRESSIVE STRENGTH OF THE CONCRETE MASONRY ASSEMBLY (f/m) SHALL BE 1500 PSI.

MINIMUM UNIT COMPRESSIVE STRENGTH = 1900 PSI. THE SPECIFIED DESIGN COMPRESSIVE STRENGTH OF THE CONCRETE MASONRY ASSEMBLY (f'm) SHALL BE 1500 PSI.

MORTAR SHALL MEET ASTM SPECIFICATION C270 FOR TYPE "S" MORTAR.

GROUT SHALL MEET ASTM SPECIFICTION C476 AND HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI.
 GROUT PLACED BY THE LOW LIFT GROUTING METHOD SHALL BE MECHANICALLY CONSOLIDATED USING A VIBRATOR WITH A MAXIMUM 3/4 INCH DIAMETER HEAD. REFERENCE THE CONCRETE UNIT MASONRY

SPECIFICATION FOR HIGH LIFT GROUTING PROCEDURES.

5. HORIZONTAL JOINT REINFORCEMENT SHALL BE LADDER TYPE (REFERENCE THE CONCRETE UNIT MASONRY SPECIFICATION). JOINT REINFORCEMENT SHALL BE SPACED AT 8 INCHES ON CENTER BELOW FINISHED FLOOR AND IN PARAPETS, AND 16 INCHES ON CENTER ABOVE FINISHED FLOOR.

6. CONCRETE MASONRY SHALL BE LAID IN RUNNING BOND.

THE CELLS FULLY GROUTED. CONCRETE MASONRY ABOVE FINISHED FLOOR SHALL BE NORMAL WEIGHT OR LIGHT WEIGHT AND IS TO BE GROUTED ONLY AT REINFORCED CELLS AND BOND BEAMS. ALL CELLS WITH REINFORCING SHALL BE GROUTED SOLID.

REFER TO WALL SECTIONS AND DETAILS FOR MISCELLANEOUS BOND BEAM LOCATIONS AND EMBEDDED ITEMS. USE OPEN KNOCK OUT BOND BEAM BLOCK. DO NOT USE TROUGH TYPE BLOCKS FOR BOND

BEAMS. DO NOT CONTINUE BOND BEAM REINFORCING THROUGH CONTROL JOINTS.

CONCRETE MASONRY BELOW FINISHED FLOOR SHALL BE NORMAL WEIGHT UNITS AND SHALL HAVE ALL

REINFORCING STEEL SHALL MEET ASTM SPECIFICATION A615, GRADE 60.

ANCHORS INSTALLED IN GROUT FILLED CONCRETE MASONRY UNITS SHALL ONLY BE USED WHERE SPECIFIED IN THE CONTRACT DOCUMENTS. ANCHORS MUST BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS. USE HILTI HIT-HY 70 MAX ADHESIVE ANCHORING SYSTEM (OR HILTI HIT ICE ADHESIVE ANCHORING SYSTEM OR HILTI KWIK BOLT 3 EXPANSION ANCHOR). REFERENCE CONTRACT DOCUMENTS FOR ANCHOR SIZE AND EMBEDMENT. SUBSTITUTIONS TO

THE SPECIFIED ANCHORS MUST BE APPROVED BY THE ENGINEER-OF-RECORD.

1. CONSTRUCTION BRACING FOR MASONRY WALLS SHALL BE DESIGNED AND DETAILED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. MASONRY SUBMITTALS SHALL CONTAIN A LETTER, SEALED BY THE PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION, STATING DESIGN LOADS AND CRITERIA WHICH WERE USED IN BRACING DESIGN. THE BRACING DESIGN DRAWINGS SHALL BE SIGNED AND SEALED BY THE PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION AND SHALL BE ISSUED TO THE OWNER AFTER SUBMITTAL

REVIEW AND PRIOR TO STARTING MASONRY CONSTRUCTION.

2. PROVIDE GALVANIZED L4x4X3/8 STEEL LINTEL IN MASONRY VENEER AS REQUIRED. MAXIMUM CLEAR SPAN = 4'-0" AND GALVANIZED L5x5x3/8 MAXIMUM CLEAR SPAN = 8'-0". ANGLE TO BEAR ON MASONRY VENEER 8" EACH SIDE TYP., U.N.O.

STRUCTURAL STEEL

MASONRY

STRUCTURAL STEEL SHALL MEET THE FOLLOWING MINIMUM YIELD STRESS (Fy):

	YIELD	ASTM SPECIFICATION
A. W, WT SHAPES:	50 KSI	A992
B. BARS, PLATES, CHANNELS, ANGLES:	36 KSI	A36
C. SQUARE, RECTANGULAR HSS:	46 KSI	A500, GRADE B
D. ROUND HSS:	42 KSI	A500, GRADE B
E. STRUCTURAL STEEL PIPE:	35 KSI	A53, GRADE B
F. ANCHOR RODS:	36 KSI	F1554 GR 36
G. ALL-THREAD RODS:	36 KSI	A36
H. HEADED STUD ANCHORS:	65 KSI TENSILE STRESS	A108, GRADES 1010
BOLTS FOR STEEL BEAM AND COLUMN CON	NECTIONS SHALL BE 3/4-INCH	H DIAMETER ASTM A325-N

BOLTS FOR STEEL BEAM AND COLUMN CONNECTIONS SHALL BE 3/4-INCH DIAMETER ASTM A325-N HIGH-STRENGTH BOLTS UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS. ALL BOLTED CONNECTIONS ARE BEARING TYPE UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS. ALL BOLTS SHALL BE TIGHTENED SNUG TIGHT UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS. FOR PRETENSIONED OR SLIP-CRITICAL JOINTS, THE METHOD OF INSTALLATION SHALL BE TURN-OF-NUT WITH MATCH MARKING, TWIST-OFF-TYPE TENSION CONTROL BOLT ASSEMBLIES (ASTM F1852), OR DIRECT TENSION INDICATORS (ASTM F959).

WELDING SHALL MEET ANSI / AWS D1.1, STRUCTURAL WELDING CODE LATEST REVISION. ELECTRODES

SHALL BE 70 KSI, LOW HYDROGEN.

4. PROVIDE DOUBLE NUTS AND DOUBLE WASHERS FOR STEEL COLUMN ANCHOR BOLTS TO ALLOW FOR ADJUSTMENT IN BASE PLATE ELEVATION. PROVIDE 1 1/2 INCH NON-SHRINK GROUT UNDER BASE PLATE AFTER ERECTION. USE 2 1/2 INCH NON-SHRINK GROUT WHEN COLUMN ANCHOR BOLTS ARE 1 1/4 INCH DIAMETER OR LARGER. NON-SHRINK GROUT SHALL BE NON-METALLIC WITH A MINIMUM COMPRESSIVE

5. ALL CONNECTIONS NOT FULLY DETAILED IN THE CONTRACT DOCUMENTS SHALL BE DESIGNED AND DETAILED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, EMPLOYED OR RETAINED BY THE STEEL FABRICATOR. THE DESIGN AND DETAILING SHALL COMPLY WITH ALL APPLICABLE CODES AND SPECIFICATION SECTIONS.

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR INCLUDING THE COSTS FOR ALL MISCELLANEOUS STEEL SHOWN IN THE CONTRACT DOCUMENTS. THESE COSTS SHALL INCLUDE, BUT ARE NOT LIMITED TO, MISCELLANEOUS STEEL ITEMS SHOWN ON THE STRUCTURAL, ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.

7. INCLUDE AN ALLOWANCE IN THE BID PRICE FOR 1500 POUNDS OF MISCELLANEOUS STRUCTURAL STEEL

INCLUDE, BUT IS NOT LIMITED TO: MATERIAL, DETAILING, FABRICATION, SHIPPING, INSTALLATION, OVERHEAD AND PROFIT.

8. PROVIDE L5x3x1/4 (LLV) FIELD FABRICATED FRAMES BETWEEN JOISTS AT OPENINGS IN ROOF THAT ARE

TO BE FABRICATED AND PLACED AS DIRECTED BY THE ARCHITECT OR ENGINEER. ALLOWANCE IS TO

PROVIDE L5x3x1/4 (LLV) FIELD FABRICATED FRAMES BETWEEN JOISTS AT OPENINGS IN ROOF THAT ARE GREATER THAN 10"x10", U.N.O., (INCLUDING ROOF DRAINS, EXHAUST FANS, RE: SF-300, AND SC-300 FOR OPENINGS AT RTU'S AND ROOF HATCHES).

STEEL JOISTS (AND JOIST GIRDERS)

STRENGTH OF 5,000 PSI AT 28 DAYS.

STEEL JOISTS (AND JOIST GIRDERS) SHALL BE AS INDICATED ON THE PLANS AND SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI) AND MEET THE

A. JOISTS SHALL BE DESIGNED FOR THE UNIFORM LOAD CAPACITY (AS SPECIFIED IN THE SJI STANDARD LOAD TABLES) IN ADDITION TO THE CONCENTRATED LOADS SHOWN IN THE CONTRACT DOCUMENTS.

B. JOISTS THAT SUPPORT CONCENTRATED LOADS SHALL HAVE THEIR CHORDS DESIGNED TO WITHSTAND ALL BENDING STRESSES, OR THE LOADS SHALL OCCUR WITHIN 3 INCHES OF JOIST PANEL POINTS, OR THE JOIST SHALL BE REINFORCED PER THE "JOIST REINFORCING DETAIL" SHOWN HEREIN. CONCENTRATED LOADS SHALL BE CENTERED ON JOISTS AND NOT ATTACHED TO THE EDGE OF CHORD ANGLES.

C. JOISTS (AND JOISTS GIRDERS) SHALL RESIST THE NET UPLIFT PRESSURE AS INDICATED ON THE "ROOF (NET UPLIFT)" SECTION OF THE DESIGN PARAMETERS FOR "DESIGN WIND PRESSURE ON COMPONENTS AND CLADDING". THIS PRESSURE SHALL ACT ALONE. AN ALLOWABLE STRESS INCREASE IS NOT PERMITTED.

D. FOR ALL MEMBERS THAT REQUIRE SPECIFIC ORIENTATION, PROVIDE TAG AT ONE END AND DEFINE LOCATION OF TAGGED END ON ERECTION DRAWINGS.

E. JOIST (AND JOIST GIRDER) MANUFACTURER SHALL DETERMINE THE SEAT DEPTH AND WIDTH OF BEARING AND COORDINATE THE SAME WITH THE STEEL FABRICATOR. THE FOLLOWING SEAT DEPTHS ARE ASSUMED IN THE CONTRACT DOCUMENTS: 2 1/2 INCH FOR K-SERIES JOISTS (5 INCH FOR LH AND DLH SERIES JOISTS) (7 1/2 INCH FOR JOIST GIRDERS).

2. K-SERIES JOISTS SHALL BE WELDED TO SUPPORTING STEEL WITH MINIMUM 1/8 INCH FILLET WELDS 2 INCHES LONG EACH SIDE OR WITH TWO 1/2 INCH DIAMETER ASTM A307 BOLTS OR THE EQUIVALENT, UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS. WHEN NEAR OR AT A COLUMN, BOLT JOIST TO SUPPORTING STEEL IN CONFORMANCE WITH OSHA. LH AND DLH-SERIES JOISTS SHALL BE WELDED TO SUPPORTING STEEL WITH MINIMUM 1/4 INCH FILLET WELDS 2 INCHES LONG EACH SIDE OR WITH TWO 3/4 INCH DIAMETER ASTM A307 BOLTS OR THE EQUIVALENT, UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS.

3. JOIST BRIDGING AND ERECTION STABILITY SHALL BE PROVIDED IN ACCORDANCE WITH THE OCCUPATIONAL SAFETY AND HAZARD ADMINISTRATION (OSHA) AND THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI).

JOIST RTU LOADS ARE PROVIDED ON THE ROOF FRAMING PLAN, REFERENCE CONTRACT DOCUMENTS FOR LOAD LOCATIONS, VALUES AND SUPPORT FRAMING.
 JOIST MANUFACTURER SHALL DESIGN THE COMPRESSION CHORD OF ALL JOISTS SUPPORTING ROOF TOP UNITS, SKY LIGHTS, AND OTHER STRUCTURES FOR AN UNBRACED LENGTH APPLICABLE TO THE CONDITIONS AT THE PROJECT WHERE THE UNBRACED LENGTH IS GREATER THAN THE SJI MAXIMUM. (REFERENCE ARCHITECTURAL AND MECHANICAL DRAWINGS)
 DESIGN JOISTS FOR INTERNAL ROOF DRAINLINE LOCATIONS, IF REQUIRED. ADD 50 PLF FOR 8 INCH

(REFERENCE ARCHITECTURAL AND MECHANICAL DRAWINGS)
DESIGN JOISTS FOR INTERNAL ROOF DRAINLINE LOCATIONS, IF REQUIRED. ADD 50 PLF FOR 8 INCH DIAMETER AND SMALLER, ADD 75 PLF FOR 10 INCH DIAMETER, ADD 102 PLF FOR 12 INCH DIAMETER, ADD 122 PLF FOR 14 INCH DIAMETER, ADD 200 PLF FOR 18 INCH DIAMETER. REFERENCE MECHANICAL AND PLUMBING DRAWINGS FOR EXACT LOCATION.
JOIST DESIGNS SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE

THE PROJECT IS LOCATED, EMPLOYED OR RETAINED BY THE JOIST MANUFACTURER.

8. SHOP DRAWING SHALL BE REVIEWED BY THE ARCHITECT AND THE ENGINEER-OF-RECORD PRIOR TO JOIST FABRICATION.

STEEL DEC

3000 PSI

ROOF DECK

A. ROOF DECK SHALL BE GALVANIZED 1.5 INCH TYPE "B" OR 3 INCH TYPE "N". DEPTH SHALL BE AS SHOWN IN THE CONTRACT DOCUMENTS.
B. ROOF DECK IS REQUIRED TO ACT AS A DIAPHRAGM. CONNECTIONS SHALL BE IN ACCORDANCE WITH STEEL DECK INSTITUTE SPECIFICATIONS. REFER TO THE ROOF DIAPHRAGM CONNECTION DIAGRAM FOR ATTACHMENT.
C. DECKING SHALL BE CONTINUOUS OVER A MINIMUM OF (3) SPANS UNLESS NOTED OTHERWISE IN THE CONTRACT DOCUMENTS.

D. NO HANGING LOADS SHALL BE ATTACHED TO ROOF DECK.
FLOOR DECK
A. COMPOSITE FLOOR DECK SHALL BE GALVANIZED COMPOSITE STEEL DECK. DEPTH SHALL BE AS SHOWN IN THE CONTRACT DOCUMENTS.
B. CONNECTIONS SHALL BE IN ACCORDANCE WITH STEEL DECK INSTITUTE SPECIFICATIONS AND AS FOLLOWS:

RE: ROOF FRAMING PLAN FOR DECK ATTACHMENT REQUIREMENTS.
 DECKING SHALL BE CONTINUOUS OVER A MINIMUM OF (3) SPANS, UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS.

GENERAL NOTES

COLD FORMED METAL FRAMING

ANY METAL STUD WALLS SHALL BE SPACED AT A MAXIMUM OF 16" ON CENTER (U.N.O.) AND SHALL BE CONSTRUCTED OF STEEL "C" STUDS OF THE SIZE SHOWN IN THE PLANS. MINIMUM WIDTH OF THE STUDS SHALL BE 1 5/8" AND THE LIP OF THE "C" PORTION SHALL BE A MINIMUM OF 1/2". YIELD STRENGTH SHALL BE AS FOLLOWS: 33 KSI FOR 18 GAGE AND LIGHTER, 50KSI FOR 16 GAGE AND HEAVIER. MINIMUM GROSS SECTION PROPERTIES SHALL BE BASED ON UNIMAST CSJ SECTIONS.

WALL STUDS AS BACKING TO MASONRY VENEER SHALL SHALL HAVE A MINIMUM THICKNESS OF 43 MILS (18 GA).
 METAL FRAMING SHALL BE IN ACCORDANCE WITH THE FOLLOWING, UNLESS NOTED OTHERWISE:

 A. 54 MILS (16 GA) AND HEAVIER
 ASTM A1003, GRADE 50 TYPE H (ST50H)

B. 43 MILS (18 GA) AND LIGHTER ASTM A1003, GRADE 30 117FE H (\$150H)
C. ACCESSORIES, TRACK AND OTHER MEMBERS ASTM A1003, GRADE 33 TYPE H (\$T33H), MINIMUM
DO NOT WELD 33 MILS (20 GA) AND LIGHTER FRAMING, UNLESS SPECIFICALLY NOTED IN THE CONTRACT DOCUMENTS.

METAL FRAMING SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN RECOMMENDATIONS. HORIZONTAL BRACING FOR WALL STUDS SHALL BE PLACED AT 48 INCHES ON CENTER OR AS PER MANUFACTURER'S WRITTEN RECOMMENDATIONS IF LESS THAN 48 INCHES ON CENTER. HORIZONTAL BRIDGING FOR JOISTS SHALL BE PLACED AT 8'-0" ON CENTER OR AS PER MANUFACTURER'S WRITTEN RECOMMENDATIONS IF LESS THAN 8'-0" ON CENTER. APPLIED FINISH MATERIALS SHALL NOT BE CONSIDERED BRIDGING OR FLANGE BRACING UNLESS NOTED OTHERWISE IN

THE CONTRACT DOCUMENTS.

6. ALL AXIALLY LOADED WALL STUDS SHALL HAVE FULL FLANGE BEARING AGAINST UPPER AND LOWER TRACK WEB PRIOR TO ATTACHMENT TO TRACK. SPLICES IN AXIALLY LOADED WALL STUDS ARE NOT

TRACK SHALL BE 54 MILS (16 GA) MINIMUM FOR WALL STUDS 54 MILS (16 GA) OR LIGHTER. TRACK SHALL MATCH WALL STUD THICKNESS FOR WALL STUDS 68 MILS (14 GA) AND HEAVIER. TRACKS SHALL BE ANCHORED AS FOLLOWS:

TO STEEL - HILTI X-U, 0.157 INCH DIAMETER KNURLED SHANK FASTENERS AT 12 INCHES ON CENTER (ESR-2269) OR APPROVED EQUAL, UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS.

TO CONCRETE - HILTI X-U. 0.157 INCH DIAMETER KNURLED SHANK FASTENERS AT 8 INCHES ON CENTER.

CONTRACT DOCUMENTS.

10. CONNECTIONS SHALL CONSIST OF ANY OF THE FOLLOWING AS NOTED IN THE CONTRACT DOCUMENTS:
A. SELF-DRILLING SCREWS OF TYPE AND SIZE AS SHOWN IN THE CONTRACT DOCUMENTS.
B. WELDS SHALL BE PERFORMED BY OPERATORS QUALIFIED IN ACCORDANCE WITH SECTION 6.0 OF AWS

WITH 1 1/2 INCH EMBEDMENT (ESR-2269) OR APPROVED EQUAL, UNLESS NOTED OTHERWISE IN

DEFERRED STRUCTURAL SUBMITTALS

D1.3, SHEET METAL.

1. THE FOLLOWING STRUCTURAL COMPONENTS SHALL BE DESIGNED AND SUBMITTED BY OTHERS FOR APPROVAL IN ACCORDANCE WITH THE

A. STOREFRONT AND CURTAINWALL FRAMING, ACCESSORIES, AND ATTACHMENTS TO STRUCTURE.
2. DOCUMENTS FOR DEFERRED STRUCTURAL SUBMITTAL ITEMS SHALL BE DESIGNED, SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER-OF-RECORD WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL AS REQUESTED WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED FOR DESIGN LOADS AND BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN CRITERIA OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL

SPECIAL INSPECTIONS

THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS PER SECTION 1704 OF THE IBC. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN THE PROJECT SPECIFICATIONS.

SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO COMPLETION OF THAT PHASE OF WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE

THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SPECIAL INSPECTOR REGARDING INDIVIDUAL INSPECTION FOR ITEMS LISTED ON THE
STATEMENT OF SPECIAL INSPECTIONS AND AS NOTED ON THE BUILDING DEPARTMENT APPROVED PLANS. ADEQUATE NOTICE AND ACCESS TO
APPROVED PLANS SHALL BE PROVIDED SO THAT THE SPECIAL INSPECTOR HAS TIME TO BECOME FAMILIAR WITH THE PROJECT.
 FABRICATORS OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1704.2 OF THE

STRUCTURAL OBSERVATION REQUIREMENTS (SAFE ROOM ONLY)

DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

A REPRESENTATIVE OF THE ENGINEER OF RECORD WILL PERFORM THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES AND AT COMPLETION OF THE STRUCTURAL SYSTEM. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTION REQUIRED OF THE BUILDING OFFICIAL OR THE SPECIAL INSPECTOR.

2. A PRE-CONSTRUCTION MEETING SHALL BE HELD AND ATTENDED BY THE ARCHITECT, ENGINEER OF RECORD, GENERAL CONTRACTOR, SUBCONTRACTORS, AND SPECIAL INSPECTORS.

3. THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD (or name of other registered design professional or firm employed by the owner) AT LEAST 48 HOURS PRIOR TO COMPLETING CONSTRUCTION OPERATIONS THAT REQUIRE STRUCTURAL OBSERVATION (BY CALLING (918) 584-5858 TO SCHEDULE A SITE VISIT.)

KIPS PER SQUARE INCH

4. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL OBSERVER:

A. AFTER INSTALLATION OF FIRST FOUNDATION REINFORCING AND BEFORE CONCRETE PLACEMENT.
B. AFTER ERECTION OF FIRST LIFT OF CMU WALL AND BEFORE GROUT PLACEMENT.
C. AFTER INSTALLATION OF CONCRETE ROOF REINFORCING AND BEFORE CONCRETE PLACEMENT.

ABBREVIATIONS ABBREVIATIONS

		KSI	KIPS PER SQUARE INCH
A.B.	ANCHOR BOLTS	L	LENGTH
A.F.F.	ABOVE FINISHED FLOOR	L.F.H.	LONG FACE HORIZONTAL
ACI	AMERICAN CONCRETE INSTITUTE	LBS.	POUNDS
ADJ.	ADJUST	LG.	LONG
AESS	ARCHITECTURALLY EXPOSED	LL	LIVE LOAD
ALOO	STRUCTURAL STEEL	LLH	LONG LEG HORIZONTAL
ARCH.	ARCHITECTURAL		
B.L.	BLOCK LINTEL	LLV	LONG LEG VERTICAL
		LONG.	LONGITUDINAL
B.O.	BOTTOM OF	MAX.	MAXIMUM
B.O.D.	BOTTOM OF DECK	MCJ	MASONRY CONTROL JOINT
BAL.	BALANCE	MECH.	MECHANICAL
BLDG.	BUILDING	MFR.	MANUFACTURER
BOT.	BOTTOM	MIN.	MINIMUM
BRG.	BEARING	MISC.	MISCELLANEOUS
C.J.	CONTRACTION JOINT	MTL.	METAL
C.L.	CENTER LINE	N.I.C.	NOT IN CONTRACT
CFMF	COLD FORMED METAL FRAMING	N.S.	NEAR SIDE
CLR.	CLEAR	N.T.S.	NOT TO SCALE
CMU	CONCRETE MASONRY UNIT	NO.	NUMBER
COL.	COLUMN		
CONC.	CONCRETE	O.C.	ON CENTER
	CONSTRUCTION	O.D.	OUTSIDE DIAMETER
CONST.		O.H.	OPPOSITE HAND
CONT.	CONTINUOUS	P.A.F.	POWER ACTUATED FASTENER
D.B.A.	DEFORMED BAR ANCHOR	PCF	POUNDS PER CUBIC FOOT
DIA.	DIAMETER	PL	PLATE
DIAG.	DIAGONAL	PLF	POUNDS PER LINEAR FOOT
DTL.	DETAIL	PMEJ	PREMOLDED EXPANSION JOIN
DWG.	DRAWING	PSF	POUNDS PER SQUARE FOOT
E.A.	EACH	PSI	POUNDS PER SQUARE INCH
E.F.	EACH FACE	QTY.	QUANTITY
E.J.	EXPANSION JOINT	R.O.	ROUGH OPENING
E.O.D.	EDGE OF DECK	RE:	REFER
E.O.S.	EDGE OF SLAB	REINF.	REINFORCING
E.W.	EACH WAY	REQD.	REQUIRED
ELEC.	ELECTRICAL		
ELEV. OR EL.		RTU	ROOF TOP UNIT
		S.D.S.	SELF-DRILLING SCREWS
EQ.	EQUAL	SCHED.	SCHEDULE
EXIST.	EXISTING	SIM.	SIMILAR
F.F.E.	FINISHED FLOOR ELEVATION	SJI	STEEL JOIST INSTITUTE
F.S.	FAR SIDE	SPECS.	SPECIFICATIONS
FDN.	FOUNDATION	STD.	STANDARD
FLR.	FLOOR	STL.	STEEL
FTG.	FOOTING	Т	THICK
G.B.	GRADE BEAM	T&B	TOP AND BOTTOM
G.C.	GENERAL CONTRACTOR	T.O.	TOP OF
GA.	GAGE	T.O.C.	TOP OF CONCRETE
GALV.	GALVANIZED	T.O.P.	TOP OF PIER
H	HEIGHT		
H.S.A.	HEADED STUD ANCHOR	T.O.W.	TOP OF WALL
		TL	TOTAL LOAD
HORIZ.	HORIZONTAL	TRANS.	TRANSVERSE
IBC	INTERNATIONAL BUILDING CODE	TYP.	TYPICAL
INFO.	INFORMATION	U.N.O.	UNLESS NOTED OTHERWISE
J.B.E.	JOIST BEARING ELEVATION	VERT.	VERTICAL
JT.	JOINT	W	WIDE
K	UNIT OF 1,000 POUNDS (KIP)	W.P.	WORK POINT
		\\/\\/ 	WELDED WIDE EVEDIC

W.W.F.

WT.

WELDED WIRE FABRIC

WEIGHT

		REQUIRED VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC
		STEEL CONSTRUCTION (IBC TABLE 1704.3)		
•	A.	MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS: IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.		Х
	B.	MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED. INSPECTION OF HIGH STRENGTH BOLTING:		Х
	A.	SNUG-TIGHT JOINT.		Х
	B.	PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION.		X
	C.	PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING OR CALIBRATED WRENCH METHODS OF INSTALLATION. MATERIAL VERIFICATION OF STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:	Х	
	_	FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.		X
	B.	FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.		Х
	C.	MANUFACTURER'S CERTIFIED TEST REPORTS.		X
	A.	MATERIAL VERIFICATION OF WELD FILLER MATERIALS: IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION		Х
	R	DOCUMENTS. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.		X
-	D.	INSPECTION OF WELDING:		
	A.	STRUCTURAL STEEL AND COLD-FORMED STEEL DECK: 1) COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS.	 X	
		2) MULTIPASS FILLET WELDS.	X	
		3) SINGLE-PASS FILLET WELDS > 5/16"	X	
		4) PLUG AND SLOT WELDS. 5) SINGLE-PASS FILLET WELDS < OR = 5/16"		X
	Г	6) FLOOR AND ROOF DECK WELDS.		X
	В.	REINFORCING STEEL: 1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706		X
		2) SHEAR REINFORCEMENT	Х	
		3) OTHER REINFORCING STEEL. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE		X
		DETAILS SUCH AS BRACING AND STIFFENING.		X
		MEMBER LOCATIONS. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.		X
		CONCRETE CONSTRUCTION (IBC TABLE 1704.4)		
		INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.		X
		INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 5B.		
•		INSPECTION OF BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	X	
		INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE.		X
		VERIFYING USE OF REQUIRED DESIGN MIX. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM	 X	X
		SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	V	
		INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.		X
0.		VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		X
l.		MASONRY CONSTRUCTION - LEVEL 1 (IBC TABLE 1704.5.1)		
		COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE		X
-		APPROVED SUBMITTALS SHALL BE VERIFIED. VERIFICATION OF I'M AND I'aac PRIOR TO CONSTRUCTION EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE.		Х
•		VERIFICATION OF SLUMP FLOW AND VSI AS DELIVERED TO THE SITE FOR SELF-CONSOLIDATION GROUT. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:	X	
		PROPORTIONS OF SITE-PREPARED MORTAR. CONSTRUCTION OF MORTAR JOINTS.		X
	C.	LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES. DURING CONSTRUCTION THE INSPECTION PROGRAM SHALL VERIFY: SIZE AND LOCATION OF STRUCTURAL ELEMENTS.		X
		TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO		X
		STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT, ANCHOR BOLTS, PRESTRESSING TENDONS AND ANCHORAGES.		Х
		WELDING OF REINFORCING BARS. PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE	X 	 X
i .		BELOW 40 DEG. F) OR HOT WEATHER (TEMPERATURE ABOVE 90 DEG. F.) PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:		
		GROUT SPACE IS CLEAN PLACEMENT OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES.		X
	C.	PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.		Χ
	D.	CONSTRUCTION OF MORTAR JOINTS. GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE.	 X	X
		PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.		Х
		REQUIRED VERIFICATION AND INSPECTION OF SOILS (IBC TABLE 1704.7)		
		VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.		Х
		VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.		Х
-		PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND	 X	X
		COMPACTION OF COMPACTED FILL. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.		X

STATEMENT OF SPECIAL INSPECTIONS (IBC 2009)

ANYTOWN PUBLIC SCHOOLS

2014 BOND - FIELD HOUSE

REVISIONS
REV. DATE DESCRIPTIO

PROJ. MANAGER:

DRAWN BY:

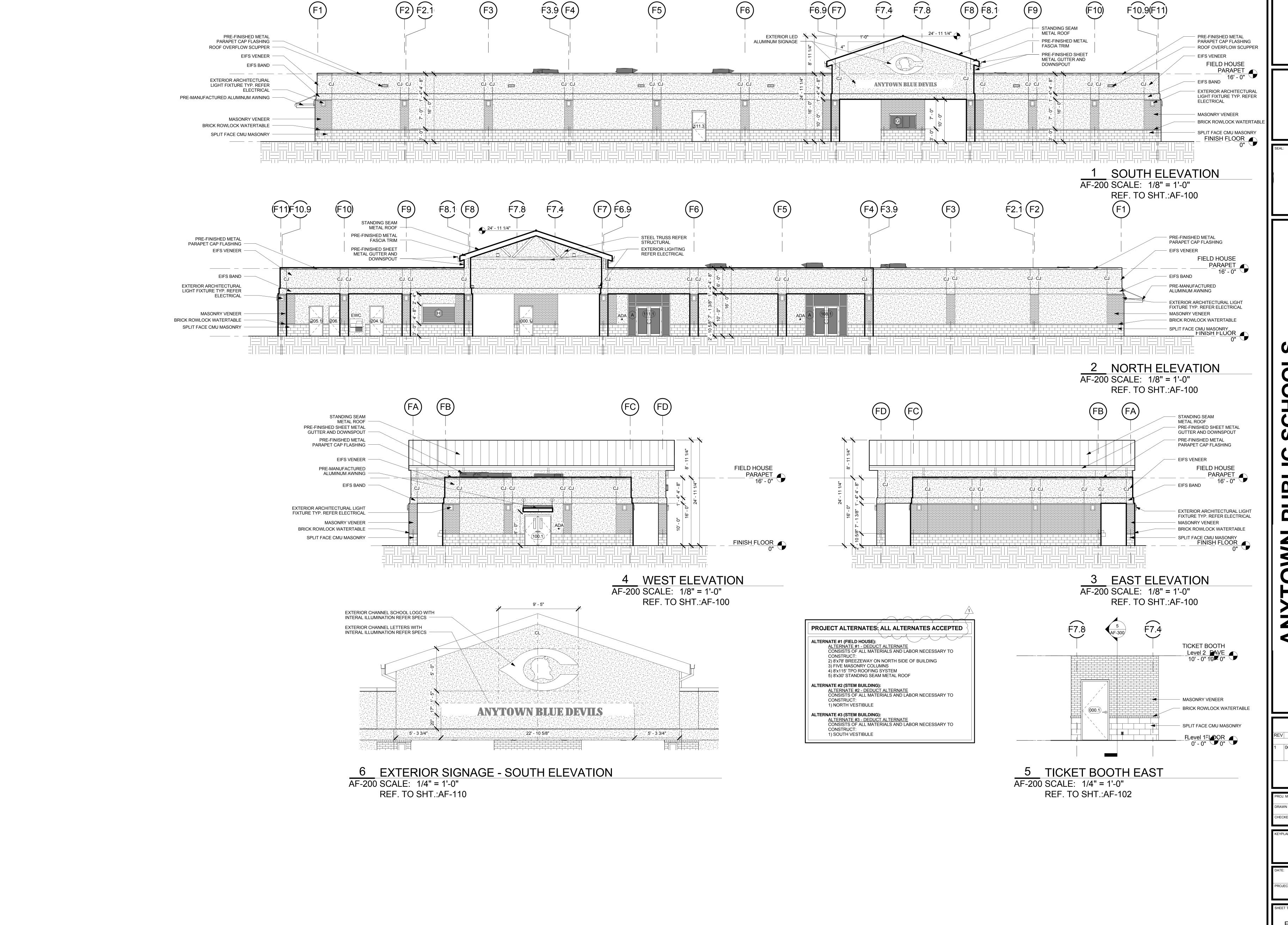
CHECKED BY:

TE:
APR. 22, 2015

DJECT NO.:

GENERAL NOTES

SF-000



20

REVISIONS V DATE DESCRIPTION 06/03/2015 FIRE REVISIONS

ELEVATIONS

AF-200